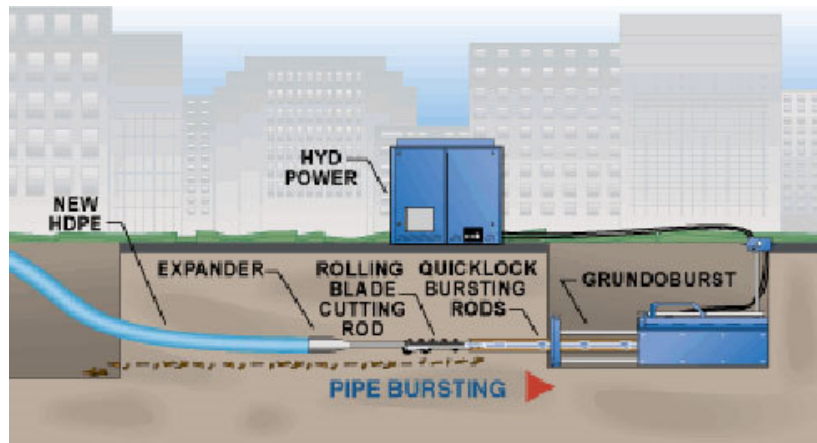


PIPE BURSTING – FINITE ELEMENT ANALYSIS FOR ESTIMATION

A. Chennakesava Reddy
 Professor, Department of Mechanical Engineering
 JNTUH College of Engineering, Hyderabad

Pipe bursting is a trenchless method of replacing buried pipelines (such as sewer, water, or natural gas pipes) without the need for a traditional construction trench. "Launching and receiving pits" replace the trench needed by conventional pipe-laying. For years, ductile iron and steel pipe has been a major limitation of pipe bursting. See how the recently introduced hydraulically operated Grundoburst pipe bursting system makes bursting ductile iron and steel pipes a problem of the past.



What is Pipe Bursting?

Replacement method involving bursting the existing pipe through brittle fracture and putting a new pipe of the same or larger size through the old fractured pipe from within.

What is Pipe Bursting?

Pipe bursting is a "trenchless" installation method that allows for minimal disruption to existing infrastructure.

History of Pipe Bursting

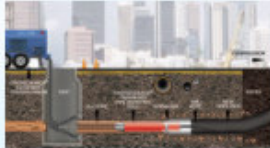
Bursting developed in the UK in late 1970's
 Method for replacement of small diameter cast iron gas mains
 By 1985, used to install 16" diameter piping
 Now lineal footage of burst pipe increasing by 20% per year, majority of this is sewers

Types of Pipe Bursting

- Static Bursting Systems
- Pneumatic Bursting Systems
- Hydraulic Bursting Systems
- Tenbusch Insertion Method
- Pipe Spitting Method

Types of Pipe Bursting

- Static Bursting Systems
- Pneumatic Bursting Systems
- Hydraulic Bursting Systems
- Tenbusch Insertion Method
- Pipe Splitting Method



Types of Pipe Bursting

- Static Bursting Systems
- Pneumatic Bursting Systems
- Hydraulic Bursting Systems
- Tenbusch Insertion Method
- Pipe Splitting Method





Figure 1-4 Hydraulic bursting head (Spandit) in expanded and contracted position


Types of Pipe Bursting

- Static Bursting Systems
- Pneumatic Bursting Systems
- Hydraulic Bursting Systems
- Tenbusch Insertion Method
- Pipe Splitting Method




Types of Pipe Bursting

- Static Bursting Systems
- Pneumatic Bursting Systems
- Hydraulic Bursting Systems
- Tenbusch Insertion Method
- Pipe Splitting Method




Types of Pipe Bursting Heads

Standard Cone Shaped Head



Types of Pipe Bursting Heads

Pneumatic Head



Types of Pipe Bursting Heads

Expanding Cone Head

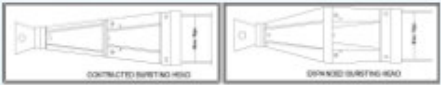



Figure 3-4 Hydraulic bursting head (Spandit) in expanded and contracted positions

Types of Pipe Bursting Heads

Splitting Head (cutting wheel or knives)



Selection of Method

Type of Existing Pipe
 Type of Soils
 Type of New Pipe
 TABLE 2.1 from ASCE MOP 112
 Discuss with Contractors

Existing Pipe	Parameters	Yes	No
Metallic pipes including aluminum, copper, ductile iron, wrought iron, steel, or stainless steel		No	Yes
Plastic pipes including HDPE or MDPE, PVC, CPVC, or Boroplast		Yes	Yes
Reinforced or non-reinforced materials including pipe (PVC or HDPE), corrugated metal pipe (CMP), or corrugated plastic pipe		No	No
Reinforced pipes, including asbestos cement (AC), RCP or non-reinforced concrete pipe, CL, VCP, or Orangeburg		Yes	Yes
Water, wastewater, storm, or sewer lines, post repair?		No	No
Flaring or pre-distributed replacement pipe		Yes	Yes
<small>Number indicates, depending on pipe and existing condition of surrounding soil. "Yes" means that this pipe may be successfully burst in these conditions. "No" means that this pipe may not be successfully burst in these conditions.</small>			

Types of Materials and Size

What types of pipe materials can be burst?
 CI, DI, VCP, AC, RCP, PVC, HDPE, Copper, etc.

What types of pipe materials can be installed?
 HDPE, PVC, DI, VCP, RCP

What are the minimum and maximum sizes that can be burst?
 Pipe bursting has been successfully completed on 4"-36" piping.

How large can you make the new pipe?
 New pipe can be upsized 2-3 sizes depending on soil conditions, new pipe material, and depth.

Advantages

- No trenching involved means minimal disruption of existing infrastructure.
- Pipe size can be increased along the same route.
- Can be more cost effective given project conditions.
- Faster installation than open cut, especially for deep pipe.
- Minimal dewatering necessary in wet conditions.
- Minimizes social costs such as traffic diversions, etc.

Disadvantages

- Must dig up lateral locations
- Cannot change slope of line
- Bypass Pumping is usually necessary
- Cannot burst through valves
- Repair sleeves or encasements may be difficult or impossible to burst
- If HDPE or welded PVC is pulled through, need room for long run of pipe
- If heaving occurs, may need some surface restoration

Planning and Design Considerations

- Soil Types and Conditions and Groundwater Depths
- Existing Pipe Material and New Pipe Material
- Surface Heaving
- Utility Locations and Connection Points
- Telesive Existing Pipeline
- PIE Locations and Pipe Layout Locations
- Cost Considerations (including Social)

City of Zeeland Case Study

Background:
 Business Growth Created Need for Increased Pipe Size, Larger Pump station

Project:
 Increase 700 LPI of VC gravity sewer from 8" to 12" by pipe burst
 Increase pump station capacity
 Increase forcemain size from 6" to 10" by open cut

Why Choose Pipe Bursting?

Utilities over the existing deep sewer pipe

- Water
- Electrical (4)
- Storm Sewer
- Gas
- Sidewalk
- Parking Lots
- Storm Pond
- Fiber Optic Cable


Why Choose Pipe Bursting?

Utilities over the existing deep sewer pipe

- Water
- Electrical (4)
- Storm Sewer
- Gas
- Sidewalk
- Parking Lots
- Storm Pond
- Fiber Optic Cable

Why Choose Pipe Bursting?

Pipe type conditions were favorable
 Deep pipe would have had large dewatering costs
 Minimal laterals to connect
 Distance (700 ft) favorable for single pull
 Directly outside of business parking lot
 Prevent Dewatering of Decorative Stormwater Pond
 Surface heave not a problem in this case



Existing Utilities and High Groundwater Levels




Excavating the Pulling Pit



Bypass Pumping & Gentex Temp Drive



Placing Whalers At Pulling Pit



Placing Thrust Blocks at Whalers



Placing Hydraulic Pulling Skid



Hydraulic Pulling Skid in Place


Capacity = 300 ton



Hydraulic Unit for Pulling Skid

A yellow hydraulic unit mounted on a skid is shown on a construction site. Two workers in safety gear are standing near the unit, which is connected to various hoses and equipment.

Connecting Pulling Rods

Two side-by-side photographs show workers in safety gear connecting long metal rods. The left image shows a worker at a site with cars in the background, while the right image shows a worker in a trench or confined space.

Pushing the Pulling Rods Through the Existing Piping

Two side-by-side photographs show the process of pushing rods through existing pipes. The left image shows a worker at a site with a large pipe, and the right image shows a close-up of the rods being inserted into a red pipe.


Excavating Manhole at Insertion Pit

Two side-by-side photographs show the excavation of a manhole. The left image shows workers in a trench with a large pipe, and the right image shows a worker in a deep excavation pit.

Layout of Long HDPE Pipe Run, DIPS, DR17

Two side-by-side photographs show the layout of a long HDPE pipe run. The left image shows a worker on a small vehicle near a pipe, and the right image shows a long pipe section laid out on a concrete structure.

Pulling Head w/ Expander

Two side-by-side photographs show the pulling of a pipe head with an expander. The left image shows a worker pulling a pipe through a hole, and the right image shows a worker in a trench pulling a pipe.


MOORE & BRIGGINK, INC.
engineering. design. construction.

Layout of Long HDPE Pipe Run, DIPS, DR17

Two side-by-side photographs showing the layout of a long HDPE pipe run, identical to the previous slide. The left image shows a worker on a small vehicle near a pipe, and the right image shows a long pipe section laid out on a concrete structure.

MOORE & BRIGGINK, INC.
engineering. design. construction.

Pulling Head w/ Expander

Two side-by-side photographs showing the pulling of a pipe head with an expander, identical to the previous slide. The left image shows a worker pulling a pipe through a hole, and the right image shows a worker in a trench pulling a pipe.



Underground service utilities in many American cities have been in place for over 100 years. While existing systems have functioned well beyond reasonably anticipated service life, underground systems are mostly deteriorated and need costly maintenance and repair. Common problems involve corrosion and deterioration of pipe materials, failure or leakage of pipe joints, and reduction of flow due to mineral deposits and debris build up inside the pipe. Damage to existing pipes can also occur by ground movements due to adjacent construction activity, uneven settlement or other ground instability. This leads to infiltration and inflow (I&I) increase in sewer systems. In water systems, it leads to flow and pressure reductions, persistent leakage (up to 30 percent of water provided in some systems), pipe bursts, and poor water quality. These problems tend to increase with the age of the network where maintaining this large network of underground sewer, water, and gas pipelines is difficult and costly. The above problems are compounded by the significant negative impacts (of open cut repair or replacement projects) on the daily life, traffic, and commerce of the area served by and along the pipeline in question.

The internal surface of the PE pipe is smoother than those of the concrete or clay pipes. For gravity applications, after some algebraic manipulation to the following Chezy-Manning equation, it can be demonstrated that the flow capacity of the PE is 44% more than those of the concrete or clay pipes considering the internal diameter for the old clay or concrete pipe equals that of the replacement PE pipe.

$$Q = \frac{1.49}{n} A (r_H)^{2/3} \sqrt{S}$$

WHERE

Q = the flow quantity

n = Manning roughness coefficient

A = the area of the pipe

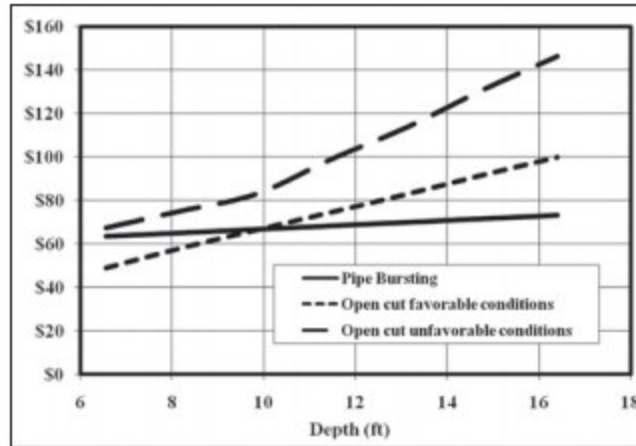
r_H = hydraulic radius

S = the slope of the energy line, which is parallel to the water surface and pipe invert if the flow is uniform.

The n value ranges for clay or concrete pipes between 0.012 and 0.015 (on average about 0.013), and it is about 0.009 for PE (Lindeburg 1992).

The increased depth has a minimal effect on the cost per foot for pipe bursting as shown in Figure (Poole et al 1985). Specific studies carried out in the US have shown that pipe

bursting cost savings are as high as 44% with an average savings of 25% compared to open cut (Fraser et al 1992). This cost saving could be much more if the soil is hard rock because rock excavation is extremely expensive compared to pipe bursting. Additionally, open cut can cause significant damage to nearby buildings and structures (Atalah 2004).



Cost Comparison Between Pipe Bursting and Open Cut Replacements (Poole et al 1985)

Summary of NASSCO Pipe Bursting Classification

Criteria	A – Routine (all of the criteria below apply)	B - Moderately Difficult to Challenging	C – Challenging to Extremely Challenging
Depth	Less than 12 feet	12 ft to 18 ft	More than 18 ft
Existing Pipe	4"-12"	12" to 20"	20"-36"
New Pipe Diameter	Size for size or one diameter upsize	Two diameter upsize	Three or more diameter upsize
Burst Length	Less than 350 feet	350 feet to 450 feet	More than 450 feet
Trench Width	Relatively wide trench compared to upsized diameter	Trench width less than 4" wider than upsized diameter	Incompressible soils (very dense sand, hard clay or rock) outside trench
Soil	Compressible soils outside trench (soft clay, loose sand)	Moderately compressible soils outside trench (medium dense to dense sand, medium to stiff clay)	Constricted trench geometry (width less than or equal to upsized diameter)

The PE pipes are available with iron pipe sized (IPS) or ductile iron pipe sized (DIPS) outside diameters. PE pipes are extruded with fixed outside diameter with variance in the inside diameter controlled by the Standard Dimensional Ratio (SDR) as shown in following equation:

$$SDR = \frac{\text{Pipe O.D.}}{\text{Wall Thickness of Pipe}}$$

The PE pipe should withstand the internal pressure requirements of the water or the force main line, overburden dead and live loads, and pulling forces during the bursting phase. The SDR of the PE pipe is a major factor in the ability of the pipe to withstand the installation forces and service pressures. Experience has shown that SDR 17 is

sufficient for gravity sewer applications, and thinner wall pipes with SDR of 19 or 21 can be used in shorter and smaller diameter applications. Thinner wall pipes tend to stretch excessively during bursting. For pressure applications, if the maximum allowable design pressure is less than 100 psi, SDR of 17 is sufficient. If the maximum allowable design pressure is more than 100 psi, the allowable pressure governs the needed SDR. If the allowable pressure is 150 psi, PE pipe with SDR 11 meets needed pressure requirements. In most trenchless applications, but not always, the pipe that withstands the pulling stresses during installation can withstand the vertical overburden and traffic pressures. The pipe stresses caused by construction are higher than those caused by vertical pressures. However, each application is different; it is possible that a specific application can require a different SDR. An engineering analysis is suggested for very deep or very shallow installations. Deep installations may be subject high overburden pressures, and shallow installations may be subject to high concentrated traffic loads that the pipe has to withstand.

Corrosion has become one of the main threats towards maintaining pipeline's integrity. At the point of corrosion, the wall of the pipe becomes thinner and starts to lose its mechanical resistance. Methods for assessing metal loss defects have been available for many decades, as for instance the NG-18 equation and ANSI/ASME B31G code. Throughout the years many modifications to the original equations have been made and newer methods like Modified B31G and RSTRENG were adopted. These days, several in-house methods and commercial codes are available but they are known to be conservative. Therefore, pipeline operators need reliable defect assessment methodology not only to assure safe operation but also to implement optimum operation cost. Based on these motivations, in the recent years various alternative methods have been developed mostly based on **finite element** studies and burst tests. This study presents the application of nonlinear **finite element** analyses for burst strength analysis of corroded pipe.

REFERENCES

1. A. Chennakesava Reddy, Safety Failure Criteria of Fluorocarbon Plastic Pipes for Dry Chlorine Transport using Finite Element Analysis, International Conference on Advancements in Materials for Manufacturing, Hyderabad, 2016.
2. Chennakesava R Alavala, Reliability computation of bursting strength of ammonia pipelines based on Choi's criterion, International Journal of Innovative Research in Science, Engineering and Technology, Vol.5, No.1, pp.28-36, 2016.
3. Chennakesava R Alavala, Dependability of bursting strength on sulphate reducing bacteria promoted corrosion of natural gas pipelines, International Journal of Scientific & Engineering Research, Vol.7, No.1, pp.51-57, 2016.
4. Chennakesava R Alavala, Influence of Chlorine Induced Corrosion and Temperature of Exothermic Reaction on Failure of Methyl Isocyanate (MIC) Storage Tanks, American Journal of Engineering Research, Vol.5, No.2, pp.1-9, 2016.
5. A. Chennakesava Reddy, Application of Eulerian and Lagrangian couplings to estimate the influence of shock pressure loading on the submersible hull using finite

element analysis, International Journal of Current Research, Vol.7, No.8, pp.19542-19547, 2015.

6. A. Chennakesava Reddy, Application of Eulerian and Lagrangian couplings to estimate the influence of shock pressure loading on the titanium submersible hull using finite element analysis, International Journal of Scientific Research, Vol.4, No.9, pp.71-75, 2015.

7. A. Chennakesava Reddy, Estimation of damage in cylinder subjected to shock pressure load using finite element analysis, International Journal of Advanced Research, Vol.3, No.8, pp.552-556, 2015.

8. A. Chennakesava Reddy, Shock Analysis of E-Glass/Epoxy Composite Submersible Hull Subjected to Pressure Loads of Underwater Explosion using Finite Element Method - Experimental Validation, International Journal of Scientific & Engineering Research, Vol.6, No.9, pp.1461-1468, 2015.

9. A. Chennakesava Reddy, Trustworthy prediction of bursting strength of ductile iron pipes based on Fitnet FSS criterion, International Journal of Research in Engineering and Technology, Vol.4, No.12, pp.48-53, 2015.

10. R. V. S. K. Varma, A. Chennakesava Reddy, Optimization of Bursting Behavior of AA2090 Al-Alloy Pipes Using Taguchi Techniques and Finite Element Analysis, International Journal of Scientific Engineering and Research, Vol.3, No.12, pp.35-38, 2015.

11. M. Akhil, A. Chennakesava Reddy, Evaluation of Structural Integrity under Bursting Conditions of Heat Treated 2219 Al-Alloy Pipes Using Finite Element Analysis, International Journal of Scientific Engineering and Research, Vol.3, No.12, pp.39-43, 2015.

12. A.Sreeteja, A. Chennakesava Reddy, Influence of Crack Size on Fracture Behavior of Heat Treated 2011 Al-Alloy Pipes Using Finite Element Analysis, International Journal of Scientific Engineering and Research, Vol.3, No.12, pp.47-50, 2015.

13. D.U.M. Manikanta, A.Chennakesava Reddy, Fracture Behavior of 6061 Al-Alloy Pipes under Bursting Loads with Crack Depth Variation, International Journal of Scientific & Engineering Research, Vol.6, No.3, pp.338-343, 2015.

14. D.U.M. Manikanta, AC Reddy, Fracture Behavior of 6061 Al-Alloy Pipes under Bursting Loads with Crack Length Variation, International Journal of Advanced Research, Vol.3, No.4, pp.657-665, 2015.

15. A. Chennakesava Reddy, Consistency prediction of bursting strength of 317 stainless steel pipes based on PCORRC (Batelle) criterion, National Conference on Excellence in Manufacturing and Service Organizations: The Six Sigma Way, pp.105-108, JNT University, Hyderabad, 2010.

16. A. Chennakesava Reddy, Reliable forecasting of remaining strength of petroleum pipelines based on LG-18 criterion, National Conference on Excellence in Manufacturing

and Service Organizations: The Six Sigma Way, pp.109-111, JNT University, Hyderabad, 2010, August

17., A. Chennakesava Reddy, Decent prophecy of bursting strength of natural gas pipelines based on modified ASME B31G criterion, National Conference on Excellence in Manufacturing and Service Organizations: The Six Sigma Way , pp.112-115, JNT University, Hyderabad, 2010.

18. A. Chennakesava Reddy, Prediction of bursting pressure of thin walled 316 stainless steel tubes based on ASME B31G criterion, National Conference on Advances in Design Approaches and Production Technologies (ADAPT-2005), pp.225-228, JNTU College of Engineering, Hyderabad, 2005.

19. A. Chennakesava Reddy, Estimation of bursting pressure of thin walled 304 stainless steel tubes based on DNV RP F101 criterion, National Conference on Advances in Design Approaches and Production Technologies (ADAPT-2005), pp.229-231, JNTU College of Engineering, Hyderabad, 2005.

20. A. Chennakesava Reddy, Reliability assessment of corrosion cracks in cold rolled 302 stainless steel tubes based on SHELL-92 criterion, National Conference on Advances in Design Approaches and Production Technologies (ADAPT-2005), pp.232-234, JNTU College of Engineering, Hyderabad, 2005.

21. A. Chennakesava Reddy, Trustworthiness judgment of corrosion cracks in cold rolled 305 stainless steel tubes based on RSTRENG criterion, National Conference on Advances in Design Approaches and Production Technologies (ADAPT-2005), pp.235-237, JNTU College of Engineering, Hyderabad, 2005.

22. B. Balu Naik, A. Chennakesava Reddy, T. Kishen Kumar Reddy, Finite element analysis of some fracture mechanisms, International Conference on Recent Advances in Material Processing Technology, Kovilpatti, pp.265-270, 2005.